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D2.4: Toward New Near-Field Ground-Motion Prediction Equations for Induced Seismicity

Abstract

Induced seismicity is currently drawing public attention as a potentially significant hazard. Several studies have been conducted to develop ground-motion prediction equations (GMPEs) for induced seismicity, however, many of them still rely on the assumption that induced events have similar source and attenuation parameters to those of natural earthquakes. We use the Preston New Road (PNR), Blackpool, UK dataset recorded between 2018-2019 with local magnitudes $M_L < 3$ at distances less than 40 km to facilitate the development of GMPEs that are tuned to the key magnitude-distance range for induced seismicity applications. The study of attenuation parameters using spectral fitting methods and coda envelope decay methods is the focus of this deliverable. Using a spectral fitting method, the best overall fit is found for a frequency-independent Q model ($\alpha = 0$) with $Q_s = 179.63$, $Q_c = 168.09$, and $Q_{sc} = 215.96$. Whereas results from a coda envelope decay method (Q_{clt}), obtained from the four biggest events recorded at PNR, show $Q_{clt}(f) = 110(f/f_{10})^{1.04}$ between 10-25 Hz. Discrepancies in the observation of Q models can be caused by the use of different methods as well as the different signal analysis windows and focus on different wavefields. Meanwhile, the difference to the average regional Q ($Q_{Lg}(f) = 266 f^{0.53}$) is likely due to different physical rock properties sampled by locally and regionally propagating waves. Through the spectral fitting approach, the high-frequency decay parameter, κ_0 , is obtained as the residual site-specific exponential decay. Observation of site condition by calculating V_{s30} using several bedrock depth assumptions and fundamental frequency (f_0) obtained from horizontal-to-vertical spectral ratios (HVSR) is also discussed in this deliverable. In addition, an updated $M_L - M_W$ relationship model for PNR dataset is presented, which shows compatibility with the $M_L - M_W$ model proposed by Edwards et al., (2019). A summary of preliminary observations is discussed in order to better understand ground motion attenuation and its controlling factors. These findings can subsequently be implemented for developing suitable GMPEs for induced earthquakes.

I. Introduction

Research interest in anthropogenic earthquakes or “induced” earthquakes has been significantly increased in recent years. Anthropogenic earthquakes can be generated by

60 underground mining, artificial water reservoir impoundment, geothermal energy production, waste
61 disposal, and hydrocarbon extraction. In spite of the fact that such events have relatively small
62 magnitude compared to natural earthquakes, it can be a significant nuisance and in extreme
63 conditions can lead to damage due to their close proximity to urban centres. In July and August
64 2014, earthquakes with magnitudes of 4.0 and 4.2 were reported near Fort St. John, British
65 Columbia, Canada. Both were considered to have been induced by hydraulic fracturing activities
66 (Atkinson et al., 2015). In the UK itself, the Human-Induced Earthquake Database (*HiQuake*) has
67 noted several earthquakes that are presumed or suspected as induced seismic events, either
68 triggered by mining, geothermal, fracking, conventional oil and gas, or construction activities
69 (Foulger et al., 2018). The largest magnitude recorded for cases found in the UK (*HiQuake*-last
70 updated on 9/6/2020) was 4.2 M_L in Folkestone, Kent, UK. Klose (2007) suggest that this event
71 may have been triggered by geo-engineering of shingle accumulation in the harbour since 1806,
72 however the evidence for this is relatively weak (Nievas et al. 2020). The most recent induced
73 event in the UK was recorded at Preston New Road with local magnitude (M_L) 2.9 (on
74 2019/08/26). This event was unequivocally caused by hydraulic fracturing during shale gas
75 exploration. Previous induced earthquakes of notable magnitude related to exploration of a shale
76 gas exploration also occurred nearby at Preese Hall on 1 April and 27 May 2011 with magnitudes
77 2.3 M_L and 1.5 M_L . Both were suspected to be linked to the hydraulic fracture injections at the
78 Preese Hall well 1 (PH1) operated by Cuadrilla Resources Ltd. (Clarke et al., 2014; de Pater and
79 Baisch, 2011).

80 In order to determine the hazard of seismic activity induced by industrial activities and also
81 develop risk mitigation, the development of ground-motion models that are well-suited for such
82 applications is required. Since the ground motion generated by anthropogenic activities certainly
83 has unique characteristics and different from those due to natural earthquakes, several challenges
84 might be found, such as: (1) the focus on, and determination of, lower magnitudes than typically
85 of interest for tectonic seismic hazard, (2) the difficulty to extrapolate directly ground-motion
86 prediction equations (GMPEs) to those small magnitudes, (3) the regional differences in ground
87 motion characteristics that become more apparent at smaller magnitude (e.g., Bommer et al.,
88 2017), and (4) the variability of motion at lower earthquake magnitude is often larger due to the
89 larger variability of stress drop compared to moderate- large events. Until recently, the prediction
90 of ground motions for induced seismicity has been done by simply borrowing the GMPEs from

91 nearby localities, similar tectonic environments, or by combining datasets. However, by simply
92 ‘borrowing’ GMPEs for induced seismicity does not often work, and combining datasets (e.g.,
93 Douglas et al., 2013) leads to unsatisfactory variance (sigma). In order to solve this problem, it
94 requires us to perform deconvolution of source, path/propagation, and site effects and develop the
95 new location-specific GMPEs designed for induced seismicity (small magnitude events at close
96 distances).

97 In this deliverable, we summarize analysis of ground motion prediction equations (GMPEs)
98 for induced seismicity due to hydraulic fracturing in Preston New Road (PNR). We also discuss
99 the efforts that have been made to observe the physical source, path, and site effects which required
100 as a starting point to better understand the ground motion characteristics for induced seismicity
101 and develop a new ground motion model specifically designed for the magnitude and distance
102 range of induced seismic events.

103

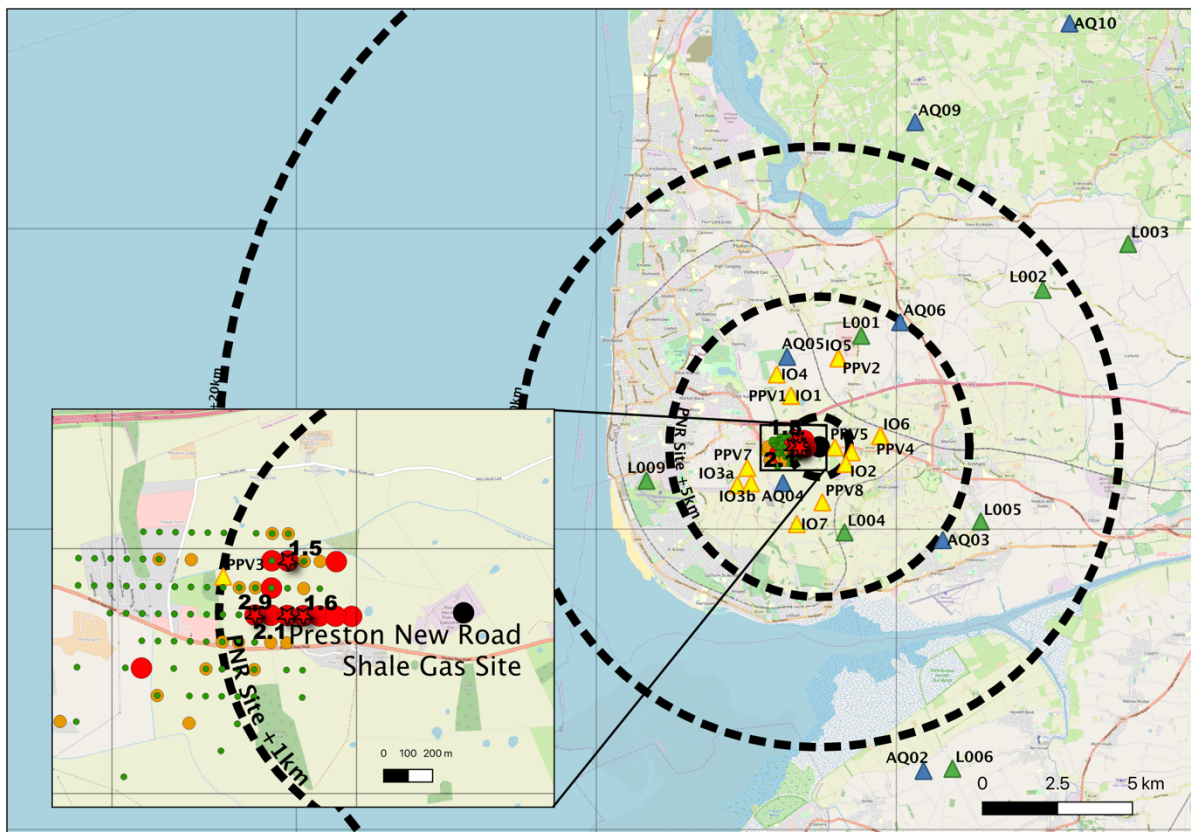
104 **II. Preston New Road Dataset**

105 Types of seismicity are typically differentiated as natural or induced. Since the UK is not near
106 a plate tectonic boundary, nor does it have any active volcanoes, the natural seismicity in the UK
107 is low compared to more seismically active regions in the world, such as Japan, Italy, and the USA.
108 However, induced seismicity has been a common occurrence in the UK. An effort to distinguished
109 between natural and anthropogenic earthquakes by determining a baseline/threshold has been
110 undertaken by Wilson et al. (2015). The study reviewed UK distribution, timing, and probable
111 causes of ~8000 onshore UK seismic events from 1970-2012 from the British Geological Survey
112 (BGS) earthquake database. They estimated that ~21% were anthropogenic, predominately caused
113 by coal mining. Up to the date of their study, two earthquakes $M_L \geq 1.5$ had been caused by
114 hydraulic fracturing. These two earth tremors, measuring 1.5 and 2.3 on the Richter scale were
115 registered at Preese Hall near Blackpool, Lancashire. They were reported to have been caused by
116 hydraulic fracturing in the area (Clarke et al., 2014). In 2018 and 2019, further hydraulic fracturing
117 was undertaken by Cuadrilla Resources Ltd. at the nearby Preston New Road site near Blackpool.

118 57 and 137 events were recorded and located in 2018 and 2019, respectively, using several
119 surface sensors operated by the British Geological Survey (BGS), Cuadrilla Resources, and
120 University of Liverpool. Earthquake magnitudes were determined using the revised M_L scale
121 developed by the BGS (Lockett et al., 2018), which extends the validity of existing UK-wide M_L

122 scale to a distance of less than 10-20 km. The dense station spacing and high-quality recordings
123 enabled detection of very low magnitude micro-seismicity below 0.0 M_L . Tens of thousands of
124 even smaller events were also detected on downhole microseismic instrumentation. The instruments
125 that were used typically record continuously in three orthogonal directions (vertical and two
126 horizontal) at sample rates of 100 or 200 samples per second.

127 According to the UK government's regulation to control induced seismicity, the Oil and Gas
128 Authority should follow the use of a traffic light system (TLS). The TLS system defines three
129 stages of action: green for normal operation, amber for magnitude between 0- 0.5 M_L , and red (M_L
130 ≥ 0.5) at which point well operation is suspended until the detailed analysis is undertaken. A 1.5
131 M_L earthquake was recorded as the biggest event in 2018 and the volume of fluid pumped was
132 reduced as a consequence. The biggest event in 2019 was 2.9 M_L was felt at the surface.



133 *Figure 1 Map showing location of seismic monitoring stations (yellow: Cuadrilla, Green: University of Liverpool, and Blue: BGS), detected seismic events based on TLS magnitude category (green dots: $M_L < 0$; orange dots: $0 < M_L < 0.5$; red dots: $0.5 < M_L$; and red stars corresponds with biggest event for 2018 (1.5 M_L at 11/12/2018), and 3 other biggest events in 2019 (on 21,24,26/8/2019 with magnitude 1.6,2.1, and 2.9 M_L respectively). Inset: zoom on epicentral region (modified after Edwards et al., 2019).*

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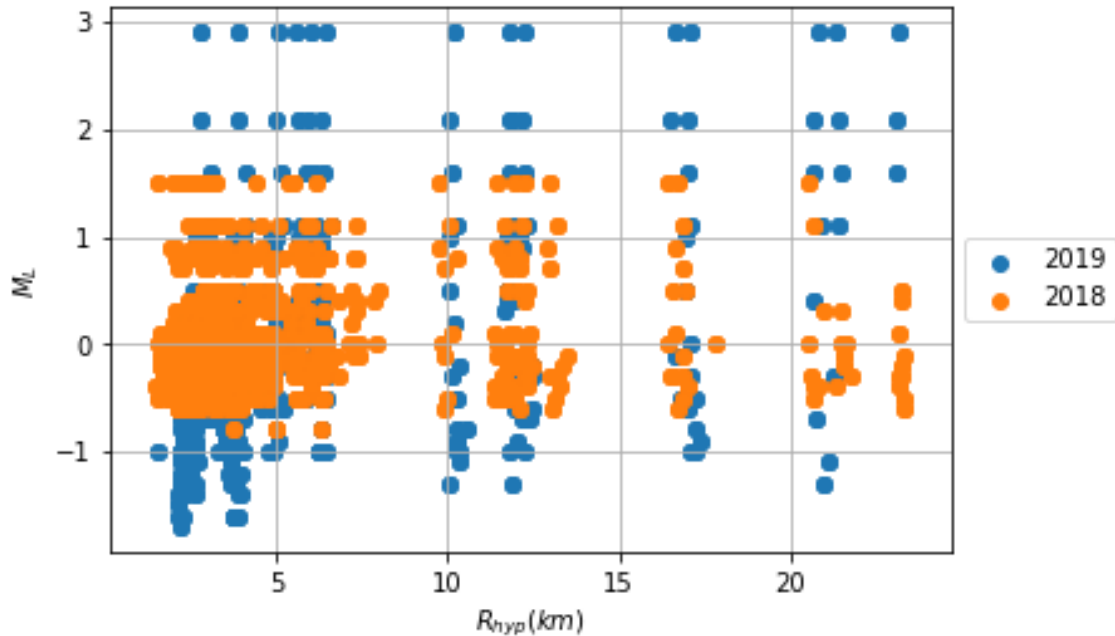


Figure 2 Magnitude-distance distribution of PNR database (2018-2019).

III. Overview of Ground Motion Models

a. Ground Motion Characteristic

Ground motion is a primary interest in seismic hazard analysis which shows the transient movements of the surface as seismic waves radiated by the earthquake passed by. Earthquake ground motion can be recorded using an accelerometer or seismometer and characterized based on the duration of shaking, point measures of peak acceleration, velocity, or displacement, frequency content, and the variability characteristic in terms of event-to-event, site-to-site and also spatial correlation. The ground motion resulting from an earthquake of a given size and distance is characterized by a predictive framework called ground motion prediction equation (GMPEs) as a simple ground motion model (GMM) that predicts the level of ground shaking and the associated uncertainty at a given location based on magnitude, distance, local site condition, etc. GMPEs for induced seismicity should provide a robust prediction for small and shallow earthquakes at close distances.

Past ground motion studies for seismic hazard in the UK have used GMPEs for stable continental regions as well as GMPEs for active crustal regions. However, since earthquakes in the UK are characterized by their comparatively small magnitudes, selection of appropriate GMPE will be tricky. Direct extrapolation for this particular small earthquakes data often leads to unsatisfactory prediction (Rietbrock et al., 2013). For example, significant deviation is observed

155 between the available recording of Groningen ground motions and predicted PGA and PGV values
156 from ground motion model developed for a neighbouring field, at Rowinkel (located 50 km away
157 in the South East from the Groningen gas field) (Dost et al., 2004). The predicted values severely
158 overpredict the recorded peaks (Bommer et al., 2017). As explained by Bommer et al. (2017) the
159 main reason is a reliable ground motion model (GMM) needs to be developed specifically for the
160 Groningen area rather than borrowing from other region was because the unusual upper crustal
161 profile and the high-velocity of Zechstein salt layer above the reservoir which cause reflection and
162 refraction of seismic waves. The stochastic method has been widely used as an alternative method
163 to develop GMPEs in low seismicity regions. Rietbrock et al. (2013) derived GMPEs for the UK
164 using 126000 simulated ground motion values from earthquake magnitude between $3 \leq M_w \leq 7$ at
165 distances ranging from 1 to 300 km. The stochastic simulations were performed based on source
166 and attenuation parameters determined by Edwards et al. (2008) using earthquakes with magnitude
167 $2 \leq M_w \leq 4$. This approach has some limitations due to systematic differences in source parameters
168 from larger earthquakes and smaller events.

169 Differences in source parameter or stress drop between tectonic and induced earthquake is
170 still debateable, some of previous studies found similar range of stress drop (e.g. Huang et al.,
171 2016,2017; Zhang et al., 2016; Ruhl et al., 2017), while others not (e.g. Abercrombie & Leary,
172 1993; Hough, 2014; Hough & Page, 2015; Boyd et al., 2017). According to Hough (2014), there
173 is a significant systematic discrepancy in source properties between natural and induced events.
174 Besides that, the study explained that induced events showed smaller intensities than predicted
175 except for results at distances less than 10 km. Other studies examined that high-frequency ground
176 motions depend on stress parameters and have lower stress parameters compared to natural
177 earthquakes (Yenier and Atkinson, 2014; Atkinson, 2015; Novakovic and Atkinson, 2015; Yenier
178 et al., 2017). Therefore, a robust GMPEs specific to the region of interest is preferred for cases of
179 induce seismicity.

180 Atkinson (2015) developed a GMPEs specifically designed for the magnitude- distance range
181 of induced seismic events. This GMPE was developed primarily from Californian earthquake data
182 with magnitudes ranging between 3-6 at distances of less than 40 km, as these tend to be the
183 magnitudes and distances at which these events can be felt. The GMPEs use hypocentral distance
184 as the distance metric, which is more appropriate for small induced events than fault-based
185 distances. Atkinson (2015) noted that if ground motions from induced events are compared to

186 those from deeper natural events, the induced motions will appear higher (i.e. stronger shaking) at
187 close distances and lower at further distances, due to the effects of shallow focal depth and stress
188 parameter scaling on the ground-motion amplitudes.

189 Several important considerations for selecting GMPEs for induced seismicity has been noted
190 by Bommer and Edwards (2018), summarised as:

- 191 1. Considering the close proximity to population centres, the risk of induced seismicity typically
192 considered as smaller magnitude compared to tectonic events therefore in some cases, the
193 extrapolation of GMPEs to smaller magnitudes will lead to over-estimation of the predicted
194 amplitudes.
- 195 2. Induced seismicity tends to occur at shallow depths (< 4 km) compared to tectonic seismicity
196 (around 10 km). Due to the proximity to the surface, the ground motion will be higher than for
197 an equivalent tectonic event at greater depths. In contradiction, because of the shallow depth
198 and lower confining stress, the stress drop may be lower and causing lower ground motion
199 compared to ground motion from the deeper events.
- 200 3. Induced seismicity hazard focussed in the near epicentral region, therefore near-field prediction
201 and shallow depth sources should be taken into account by using distance measures such as
202 hypocentral or rupture distance.
- 203 4. Flattening of the attenuation curves at short distances as near-source saturation phenomenon.
204 Distance saturation is dependent on the magnitude with a flattened part of the attenuation curve
205 extending over a greater distance from the source for larger earthquakes.
- 206 5. It is important to have suitable GMPEs for the V_{s30} in the region since it will give significant
207 differences in predictions.
- 208 6. Some predictor variables give relatively minor changes in prediction and not considered as
209 important as other predictor variables described above. Therefore, it is better to assume a simple
210 model form that can be easily adjusted.

211

212 **b. Prediction of Ground Motions**

213 A number of reviews and summary about ground motion estimation studies have been made
214 in the past. The newest review by Douglas (2019) provides summary details of studies for PGA,
215 PGV and response spectra published (as found in journals, conference proceedings, technical
216 reports, and some PhD theses) between 1964 and late 2019. It summarizes, in total, 462 empirical

217 GMPEs for the prediction of PGA and 299 empirical models for elastic response spectral ordinates
 218 prediction. Another summary published by Stewart *et al.* (2015) presents a GMPE selection
 219 procedure that evaluates multidimensional ground motion trends, examines functional forms, and
 220 quantitative tests of GMPE performance for stable continental regions, subduction zones, and
 221 active shallow crustal regions. As explained above, ground motion is affected by complex process,
 222 and could be different due to the specific near surface condition, different geology structure, and
 223 earthquake mechanism itself. Therefore, ground motion might be different from one region to
 224 another. At Preston New Road, a review conducted for the UK Oil and Gas Authority (Edwards et
 225 al., 2019) focused on GMPEs from Atkinson (2015) and Douglas et al. (2013) which are commonly
 226 used for predicting ground motion due to induced seismicity.

227

228 ***Atkinson, 2015***

229 GMPEs from Atkinson (2015) were specifically developed to help evaluate seismic hazard from
 230 induced seismicity. The model is developed using events of M 3 to 6 at hypocentral distances less
 231 than 40 km from the NGA-West2 database. The model is described to the functional form:

$$\log Y = c_0 + c_1 M + c_2 M^2 + c_3 \log R + B(\tau) + W(\varphi) \quad (1)$$

232

233 where Y is the ground motion parameter, logs are in base 10, $B(\tau)$ and $W(\varphi)$ describes the
 234 between- and within-event variability, M is moment magnitude and R is an effective point-source
 235 distance that take into accounts the near-epicenter saturation of motions, expressed as:

$$R = \sqrt{(R_{hyp}^2 + h_{eff}^2)} \quad (2)$$

236

237 where R_{hyp} is the hypocentral distance and h_{eff} is near-epicenter saturation of motions. h_{eff}
 238 value used in the analysis for PNR dataset suggested by Bommer and Edwards (2018), written as:

$$h_{eff} = \max(1, 10^{-0.28+0.19M}) \quad (3)$$

239

240 The predictions were calculated using near-surface shear-wave velocity reference of 760 m/s,
 241 which corresponds to rock. The coefficients of Eq.1 denoted as c_0 , c_1 , c_2 , and c_3 were determined
 242 by Atkinson (2015) using maximum likelihood regression method. One of the advantages of this
 243 model compare to model by Douglas et al. (2013) is the simplicity of the selected functional form

244 which restrict its applicability to distance less than 50 km. In this case, the attenuation term only
245 modelled as linear in $\log R$ without curvature on the slope due to growing anelastic effects at larger
246 distances. Therefore, this formula (Eq.1) has some limitations if the objective is for regional
247 distance applications or comparison with datasets over a wider distance range.

248

249 *Douglas et al., 2013*

250 Douglas et al. (2013) investigate ground motions generated by induced earthquakes and
251 particularly those associated with geothermal or enhanced geothermal systems (EGSs).
252 Douglas et al. (2013) suggested that differences in source, path and site conditions were the likely
253 cause of region-specific differences. One model was produced without correction for site effect
254 and one with corrected to a reference rock with $V_{s30} = 1100$ m/s. The dataset included events with
255 magnitude 1 to 4 at distances up to 20 km. GMPEs by Douglas et al. (2013) formed as:

$$\ln Y = a + bM + c \ln R + dR_{hyp} + B(\tau) + W(\varphi) \quad (4)$$

256

257 where Y is the ground motion parameter, M denotes the moment magnitude, R is the point-source
258 distance explained in Eq.2, R_{hyp} is the hypocentral distance, and $B(\tau)$ and $W(\varphi)$ describe the
259 variability. Key differences from this model with Atkinson (2015) are the use of natural logarithms
260 rather than base-10, and the lack of M^2 term. Coefficients of a , b , c , and d were obtained by the
261 authors using regression analysis, with b indicating the magnitude scaling of the derived GMPEs
262 and suggesting a comparable magnitude scaling of induced, mining, and natural seismicity
263 (Douglas et al., 2013; Douglas and Jousset, 2011). The regression coefficient of c and d are non-
264 unique and may correlate one another. Both coefficients imply a fast decay with distance that
265 represent geometrical spreading and intrinsic attenuation which are difficult to be distinguished
266 clearly.

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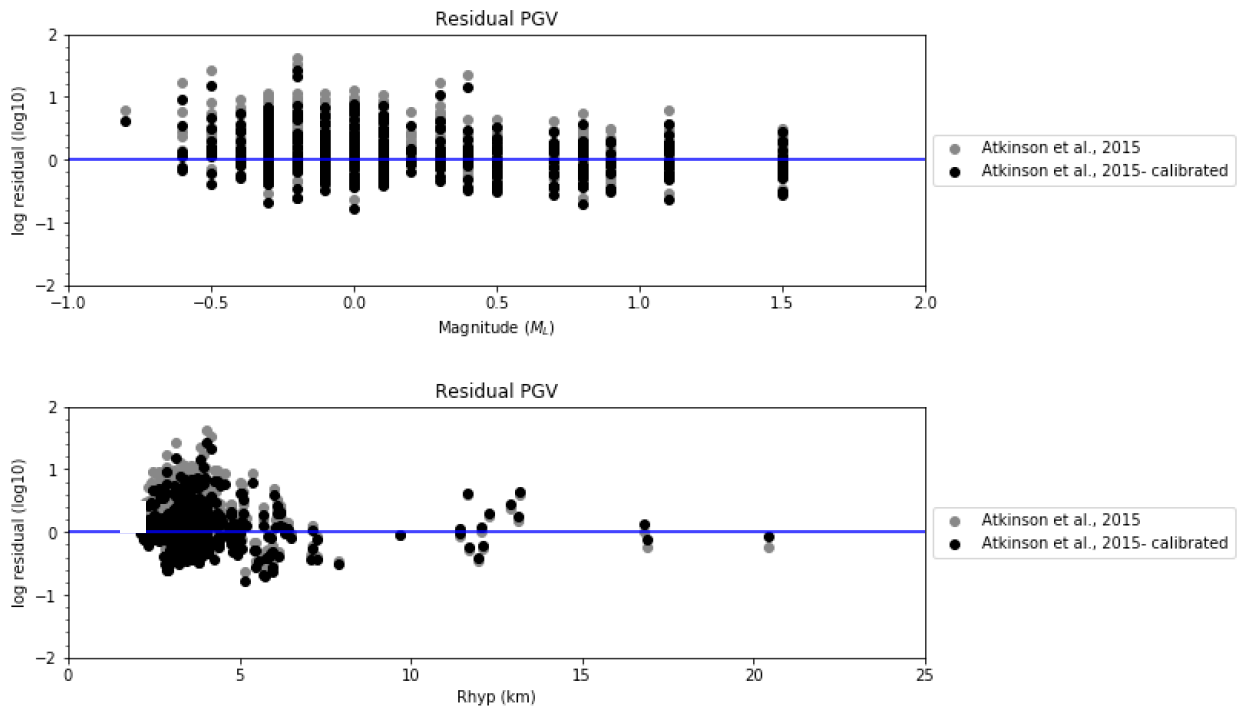
268 **c. Summary of Previous Ground Motion Predictions for PNR Dataset**

269 A previous study about ground motion models for Preston New Road (PNR) was carried out
270 by Edwards et al. (2019) adapting from Atkinson (2015) and Douglas et al. (2013), as common
271 GMPEs used for predicting ground motion from induced seismicity. Since both GMPEs developed
272 using moment magnitude (M_w) while the PNR catalogue provide magnitude as local magnitude
273 (M_L), then it is necessary to convert M_L to M_w . Two conversion models were tested by authors:

274 Grünthal et al. (2009) as M_W - M_L conversion based on tectonic of European events and Edwards
 275 et al. (2015) which is in accordance to empirical data of PNR-1z presented by Cuadrilla Resources
 276 (2019b). The assessment of GMPEs for PNR ground motions was done by comparing the residual
 277 PSA, PGA, and PGV using both GMPEs (Atkinson, 2015 and Douglas et al., 2013) along with the
 278 magnitude conversion from M_L to M_W of Grünthal et al. (2009) and Edwards et al. (2015).

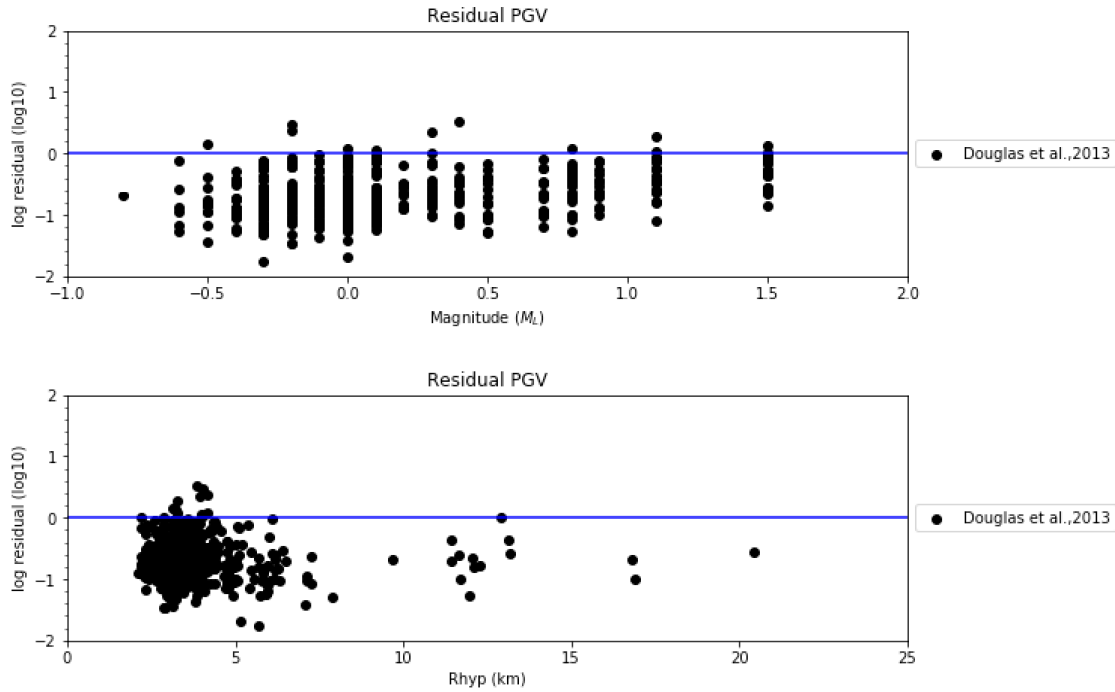
279 According to the Edwards et al (2019), the best approach for predicting ground motions from
 280 PNR-1z seismicity is to use the Atkinson (2015) with magnitude conversion based on Edwards et
 281 al. (2015). Even though GMPEs from Atkinson (2015) was developed using magnitude >3 , it
 282 performs well consider that Atkinson (2015) used a more complex functional form, including M^2
 283 term and magnitude dependent near-field saturation. The authors further note that for predicting
 284 ground motions from larger events at PNR ($M_L > 2.5$) the GMPEs from Atkinson (2015) should be
 285 used with M_L to M_W of Grünthal et al., (2009).

286



287

288 *Figure 3 Residual plot from Atkinson (2015)- M conversion using Edwards et al., (2015).*
 289 *(adapted from Edwards et al., 2019)*



291

292 *Figure 4 Residual plot from Douglas et al., (2013)- M conversion using Edwards et al., (2015).*
 293 *(adapted from Edwards et al., 2019)*

294 Interesting discussion is also found in regard to Figure 3 and 4, where residual PGV values
 295 (in log 10 unit or factor of 10) from Atkinson (2015) are positive and shows the underprediction
 296 of the observations PGV values while the Douglas model exhibit contrary behavior with
 297 overprediction PGV values. A standard deviation (σ_{total}) of 0.33 is estimated for PGV. In both
 298 figures, the residual value decreases with the increasing magnitude and distance which shows that
 299 the discrepancy of PGV values is more clearly seen at a closer distances (and to some extent at
 300 smaller magnitude). These results also indicate an unsatisfactory sigma between predicted-
 301 observed values, particularly considering the ‘single-source-zone’ nature of the induced
 302 seismicity. These disparities clearly justify additional effort to improve ground motion models for
 303 Preston New Road.

304

305 **IV. Toward New Ground Motions Prediction Equations**

306 Ground motion recorded in each station can be seen as the combination of three contributions:
 307 source characteristics, wave propagation (geometrical, intrinsic attenuation, and scattering effect),

308 and site responses. Generally, GMPEs are the simplified model which deal with all three aspects.
309 It is typical that ground motion cannot be described in detail considering the complexity of the
310 process that occurs. This is due to the lack of information (e.g., about the rupture, the crustal
311 structure and the near-surface effects) and the nature of the simplifications inherent in GMPEs.
312 Therefore, to determine the variability of ground motion affect by each aspect, the separation into
313 physical source, path, and site term will be followed in this study project.

314

315 **a. Earthquake Magnitude**

316 Source terms in ground motion models (GMMs) are typically represented by the earthquake
317 magnitude and stress drop. An earthquake's moment magnitude can be described as the
318 earthquake's 'size' and relate to physical characteristics of the fault/ crack that generated seismic
319 waves denoted as M_w . The moment magnitude (M_w) scale, is uniformly applicable to all sizes of
320 earthquakes and more directly related to the energy of an earthquake than other scales, also does
321 not saturate but it is more difficult to compute. Meanwhile, the local magnitude (M_L), also known
322 as the Richter scale, is still widely used in different part of the world, because it easier to calculate.
323 It describes the surface effects, normalized to a common reference distance, without consideration
324 of the physical source. Different magnitude scales are usually calibrated to be consistent with one
325 another. Several different models have been proposed to convert between M_L and M_w developed
326 from different type of datasets. Grünthal et al. (2009) define a relationship between M_L - M_w over
327 a wide magnitude range. Meanwhile, model proposed by Edwards et al. (2015) and Cuadrilla
328 Resources (2019b) obtained from magnitude < 2 show that moment magnitude of induced
329 earthquakes at PNR are systematically higher than local magnitude. This is consistent with
330 numerous other studies of both induced and tectonic events (e.g., Dost et al, 2018) and is due to
331 the fact that path (attenuation) effects band-limit the high frequency motions of small events
332 (Deichmann, 2017).

333 Here we present an updated model of M_L - M_w relationship for the PNR dataset based on
334 direct calculation of M_w for selected events. After removing the instrument response and applying
335 cosine taper, S-Coda wave and noise windows are defined. The signal window (in this case from
336 beginning of S-wave until the end of coda window) and noise window are transformed into the
337 frequency domain using multitaper spectral estimation techniques (Prieto et al., 2009). Henceforth,
338 records were selected based on signal-to-noise ratio ($SNR > 2$).

339 The signal displacement spectrum $A(f)$ recorded in one station can be written a product of a
 340 source term $\Omega(f)$, attenuation term $P(R, f)$, and site effect term $S(f)$:

$$A(f) = \Omega(f) * P(R, f) * S(f) \quad (5)$$

341
 342 where R is the hypocentral distance and f is the frequency. In this study, we assume the site effect
 343 is uniform therefore we neglect $S(f)$. For a source model, the Brune (1970) model is chosen,
 344 combined with the path/ attenuation term can be expressed as:

$$\Omega(f) * P(R, f) = \frac{\Omega_0 e^{-(\pi f^{1-\alpha} t^*)}}{1 + (f/f_c)^2} \quad (6)$$

345
 346 in which Ω_0 is the low- frequency plateau, f is the frequency, f_c is the corner frequency and t^* is
 347 the attenuation parameter ($t^* = T/Q_0$, with T the travel time and Q the path-average quality
 348 factor). The Ω_0 term contains geometrical spreading, radiation pattern, seismic moment, and other
 349 frequency-independent effects. An inversion process is applied to fit t^* , f_c , Ω_0 , and α (if we
 350 consider frequency-dependence of Q ; otherwise $\alpha = 0$) with the model defined in Eq. 5 and 6.

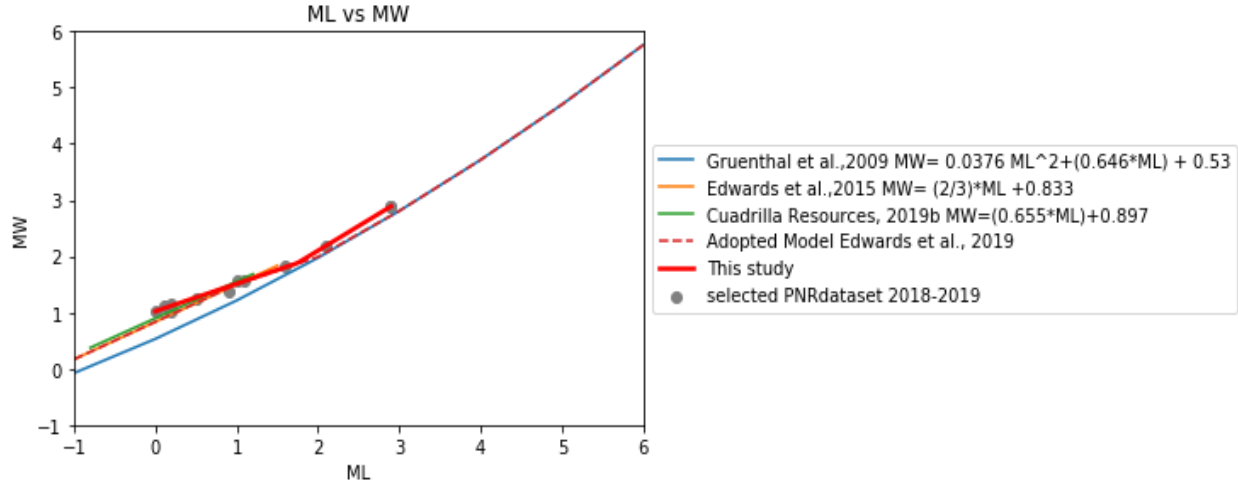
351 The seismic moment (M_0) of a seismic record can be written as:

$$M_0 = \frac{4 \pi \rho v^3 R_{hyp} \Omega_0}{F_s R_{\theta\phi}} \quad (7)$$

352
 353 where ρ is the rock density at the source (2800 kg/m^3), v is the velocity in the source ($v =$
 354 2000 m/s), R_{hyp} is the hypocentral distance, and Ω_0 is the low- frequency plateau. F_s is Free
 355 surface amplification factor ($F_s = 2$ for normally incident SH waves and a good approximation
 356 for SV) and $R_{\theta\phi}$ is the average radiation pattern coefficient for S-wave (0.55) (Boore & Boatwright,
 357 1984).

358 From the relationship between M_0 and M_W , we can calculate M_W :

$$M_W = \frac{2}{3} \log M_0 - 6.03 \quad (8)$$



359

360

Figure 5 Local to moment magnitude conversions

361

362

Bi-linear regression performed to fit the scattered M_W for selected PNR dataset, and can be summarized as:

$$M_W = k_1(M_L - 1.749) + 1.88 \quad \text{for } M_L \leq 1.749 \quad (9 \text{ a})$$

$$M_W = k_2(M_L - 1.749) + 1.88 \quad \text{for } M_L \geq 1.749 \quad (9 \text{ b})$$

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where $k_1 = 0.4907$ and $k_2 = 0.8774$ are the slopes of regression line. This new model supports the use of Edwards et al. (2019) model (dashed red line) for magnitude range between 0-3. However, if we extrapolate the new model (red line) below 0 M_L or upper 3 M_L , it will give higher M_W respected to M_W from model by Edwards et al. (2019). Therefore, we need more data to justify the reliability of this model.

b. Stress Drop

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Besides magnitude, dynamic stress drop can help us to characterize a seismic event and can be considered to represent energy release. During a seismic event, the dynamic stress drop implies a significant impact on the radiated wavefield. There are various ways to calculate stress drop, but as used in engineering seismology stress parameter effectively defines the proportion of high-frequency radiated energy for a given magnitude. Higher stress drop events emit a greater proportion of high frequency energy.

376

377

378

Stress drop defined as static measure describing the average stress acting on fault before and after rupture (Kanamori, 1977; Hanks, 1979; Boore, 1983). In this case, information regarding the size of the rupture is needed. However, for small earthquakes, direct observations of the rupture

379 geometry are not possible so the fault dimensions must be estimated from far-field observations
380 of the radiated seismic waves. Therefore, the methods of estimating stress drop for small
381 earthquakes derived under assumptions about the dynamics of the source, they are sometimes
382 termed as “dynamic stress drop” or “Brune stress drop” (Brune, 1970; Shearer, 2009; Holmgren
383 et al., 2019). Hough (2014) infers that induced earthquakes have lower stress drops than tectonic
384 earthquakes based on a comparison of non-instrumental “Did You Feel It?” intensities. Other
385 studies explained that induced earthquake sequences may have comparable stress drops to tectonic
386 earthquakes (Huang et al., 2016,2017; Zhang et al., 2016; Ruhl et al., 2017). Huang et al. (2017)
387 suggests that ground motion prediction equations developed for tectonic earthquakes can be
388 applied to induced earthquakes after properly considering the effects of depth and faulting style.
389 This argument is supported with their findings that in the strike-slip dominant area (central United
390 States) shows a comparable median stress drop of induced seismicity to the tectonic earthquakes.
391 On the other hand, in North America, which exhibits dominantly reverse faulting, a lower median
392 stress drop of induced earthquakes is observed with respect to tectonic earthquakes. Earthquake
393 stress drop also often considered depth-dependent, with deeper earthquake leads to higher stress
394 drop (Edwards et al., 2019; Huang et al., 2017). According to Huang et al. (2017), the depth
395 dependence of stress drop estimates suggests that more intense ground motions are expected from
396 deeper earthquakes for a given hypocentral distance (deeper earthquake generate higher stress
397 drop). Since GMPEs are usually developed using predominantly deep tectonic events, in some
398 cases where the stress drop of induced earthquakes is not comparable with the stress drop from
399 tectonic earthquakes, predictions will overestimate ground motions when applied to shallow
400 induced earthquakes. It is therefore important to consider both the effects of depth-dependent stress
401 drops and propagation effects for predicting ground motions of induced earthquakes.

402

403 **c. Attenuation**

404 Seismic ground motions will decrease as the increasing distance from the source, partly for
405 geometric reasons because their energy is distributed on an expanding wave front, and partly
406 because their energy is absorbed by the material they travel through. Geometrical decay is due to
407 the fact that energy must be preserved over an increasing large surface, means that amplitude must
408 be decrease proportional to distances. When seismic waves propagate beneath the surface, the
409 waves not only lose energy through geometrical spreading effects but also through intrinsic and

410 scattering attenuation. Intrinsic attenuation accounts for the seismic energy which converted into
 411 different energy types (e.g., heat), and scattering attenuation describes the redistribution of seismic
 412 energy into different directions. Usually for simplification, geometrical decay and attenuation
 413 modelled as:

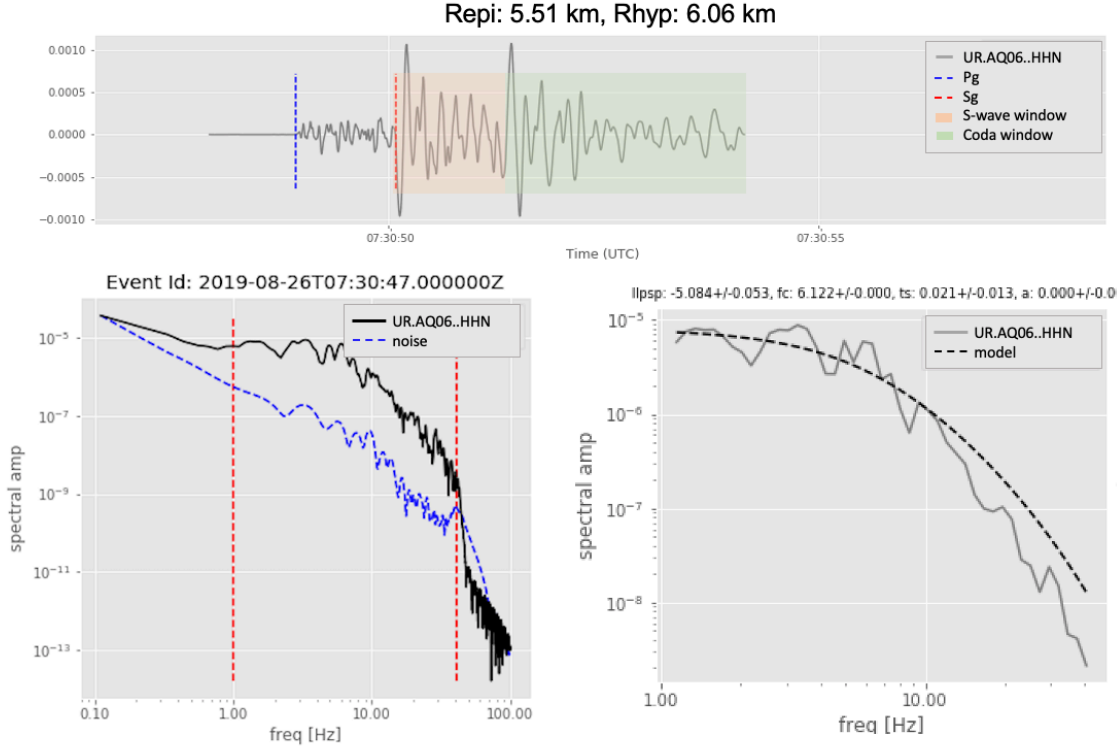
$$G(f) = \frac{\exp^{-\pi f R / \beta Q(f)}}{R^\lambda} \quad (10)$$

414 where R is the hypocentral distance, β is the average shear wave velocity, Q is the quality factor
 415 and λ is the rate of geometrical spreading. For shallow induced seismicity, near-field motions tend
 416 to decay more rapidly than $1/R$ (Edwards et al., 2019; Atkinson, 2015; Butcher et al., 2020; Ameri
 417 et al., 2020).

418 Measurements of seismic attenuation (Q^{-1}) can vary considerably when made from different
 419 part of seismograms or using different techniques, especially at high frequencies (Sarker & G.A.
 420 Abers, 1998). Such differences could be methodological or may reflects earth processes. In this
 421 study, the measurement was made for different signal window (S-wave, coda wave, and S-coda
 422 wave) utilizing two different approaches: (1) parametric fit to spectral decay, and (2) coda
 423 envelope decay with time. The parametric fit to spectral decay observed using 3 different signal
 424 windows mentioned above, while coda envelope decay with time was applied only for the coda
 425 wave window. For the parametric fit to spectral decay methods, attenuation along with source
 426 parameters can be inverted in a parametric scheme. A spectral fitting method was performed using
 427 horizontal component (east-west (E) and north-south (N)) of 194 events recorded in Preston New
 428 Road in 2018-2019. However, only recordings with good signal-to-noise ratio ($SNR > 3$) will be
 429 considered. Processing time series data such as removal of instrument response, detrending,
 430 tapering signal, determining noise and signal windows (S-wave, coda wave, or S-coda wave) were
 431 performed before the calculation of Fourier spectra. The spectral of chosen signal window and
 432 noise window then calculated using multitaper spectral estimations method. After that, the signal-
 433 to-noise ratio (SNR) was calculated and lower and upper frequency bound was determined. Only
 434 recordings with good quality of SNR ($SNR > 3$) will passed to the spectral fitting step. An inversion
 435 approach to fit model described in Eq. 6 was performed for four defining parameters: Ω_0 , f_c , α ,
 436 and t^* . The methods allow us to calculate both frequency-independent and frequency-dependent
 437 t^* measurements. Frequency dependent of Q is considered by parameterizing t^* as:

$$t^*(f) = t_0^* f^{1-\alpha} \quad (11)$$

438 where t_0^* represent t^* at $f=1$ Hz and α describes frequency dependency.

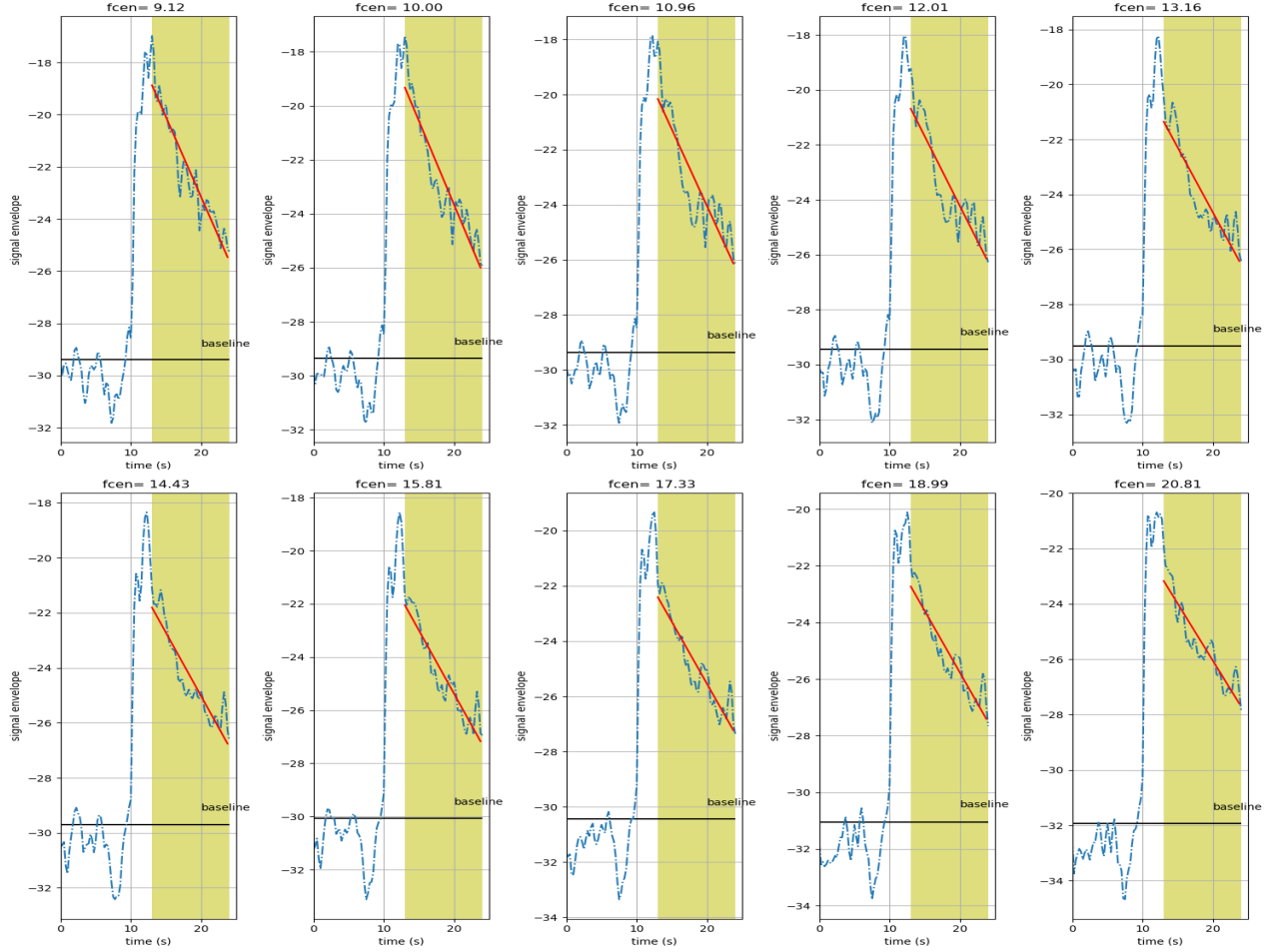


439
440 *Figure 6 Example of spectral fitting process: (Top) S-wave and coda time windows, (Bottom left)*
441 *Fourier amplitude spectra of signal and noise, and (Bottom right) spectra fitting from ML 2.9*
442 *event occurred at 26/08/2019 recorded in the north-south component of station UR.AQ06.*

443 An alternative method using coda envelope decay with time can be inverted for apparent
444 attenuation and can be done by assuming single scatter or multiple scattering model. For
445 simplicity, in this work we assume single scattering model as expressed below:

$$A(f, T) = A_0(f) T^{-\nu} e^{-\pi f T / Q_{clt}} \quad (12)$$

446 where Q_{clt}^{-1} is the coda attenuation which is assumed to vary with frequency in a manner identical
447 to t^* in Eq.11, A_0 represents source factor that includes as independent contributor for attenuation,
448 f is the frequency, T is lapse time since the time of occurrence/ earthquake origin time, and ν is a
449 positive constant that is related to geometrical spreading (Aki and Chouet, 1975), in this study
450 $\nu=1$, which represent the spherical spreading of body wave.



451

452 *Figure 7 Example of coda envelopes obtained as well as linear regression $Q_c(f)$ (red line) for*
 453 *10 different central frequencies in bands [10-25 Hz]. Noise baseline is shown as a horizontal*
 454 *black line and coda windows displayed as yellow rectangle.*

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There are some limitations in the application of Q calculation using coda envelope decay technique found in this study. Due to the shallow and short duration records available, there is a probability that coda waves are not being captured well. An adjustment has been made for defining coda lapse time (time lapsed after the origin time where the coda starts), and defined as $t_c = 1.4(t_s - t_p) + t_s$ modified from the original version proposed by Perron et al. (2017) which model the beginning of coda as $t_c = 2.3(t_s - t_p) + t_s$. The time lag between the end of S-wave and the beginning of coda is very short and may affect noise contamination and the influence of S-wave in the coda window. Smaller windows give less stable results, particularly at low frequencies. Therefore, the Q_{clt} were observed for 10 frequencies in the bands 10 – 25 Hz using four biggest events recorded from PNR dataset 2018-2019 (see Figure 1). Q_{clt} from individual recordings $Q_{clt(i)}$ were calculated using linear regression of slope of analytic signal which

466 represent decay of coda envelope (Figure 7). Collection of $Q_{clt(i)}$ from all events then are used to
 467 calculate Q_{clt} evaluated at frequency of 10 Hz ($Q_{clt}(f = 10)$ or denoted as Q_{10} in Table 1) for
 468 different sensors which provided in Table 1 with mean Q_{10} value of 113.998 and standard deviation
 469 of 36.3. Estimation of mean- Q_{10} and mean- α may represent Q_{clt} value of PNR region, which
 470 modelled as $Q_{clt}(f) = 114(f/10)^{1.2}$. Lower Q_{10} values were found around epicentre of PNR
 471 site and directed towards the coastal area, while towards the northwest of the site, Q_{10} values seem
 472 increased. Thus, high attenuation indicated by lower Q_{10} values is likely to be associated with
 473 more sediment deposits and these tend to be coastal and river based (see Figure 8). Another model
 474 of Q_{clt} produced for PNR dataset by stacking $Q_{clt(i)}$ from all events stated as $Q_{clt}(f) =$
 475 $110(f/10)^{1.04}$. Both $Q_{clt}(f)$ modelled in relatively upper crust layer (< 30 km), while the
 476 regional Q determined from multiply reflected shear waves (Lg) and correlate with large scale
 477 crustal features (up to 100 km or deeper). Therefore, obviously the local Q smaller than regional
 478 model for Britain $Q_{Lg}(f) = 266 f^{0.53}$, which is modelled between 1 – 10 Hz frequency bands for
 479 regionally propagating Lg waves (Sargeant & Ottemöller, 2009).

480 *Table 1 Q_{10} and α calculated in each station using the 4 biggest events recorded at Preston New*
 481 *Road*

Station	Q10	alpha
LV.L001	76.4	0.83
LV.L002	111.0	1.32
LV.L003	191.5	1.88
LV.L006	76.8	1.74
LV.L009	118.6	0.34
UR.AQ03	81.0	1.23
UR.AQ04	81.8	0.79
UR.AQ05	71.0	0.78
UR.AQ06	87.4	1.21
UR.AQ07	169.1	1.48
UR.AQ09	115.4	1.21

Station	Q10	alpha
UR.AQ10	183.9	1.77
SD.IO1	106.1	0.78
SD.IO2	124.3	1.19
SD.IO3A	92.7	1.75
SD.IO4	120.1	0.75
SD.IO5	130.8	1.03
SD.IO6	113.9	0.9
mean	114.00	1.17
stdv	36.29	0.43
std error	8.55	0.10

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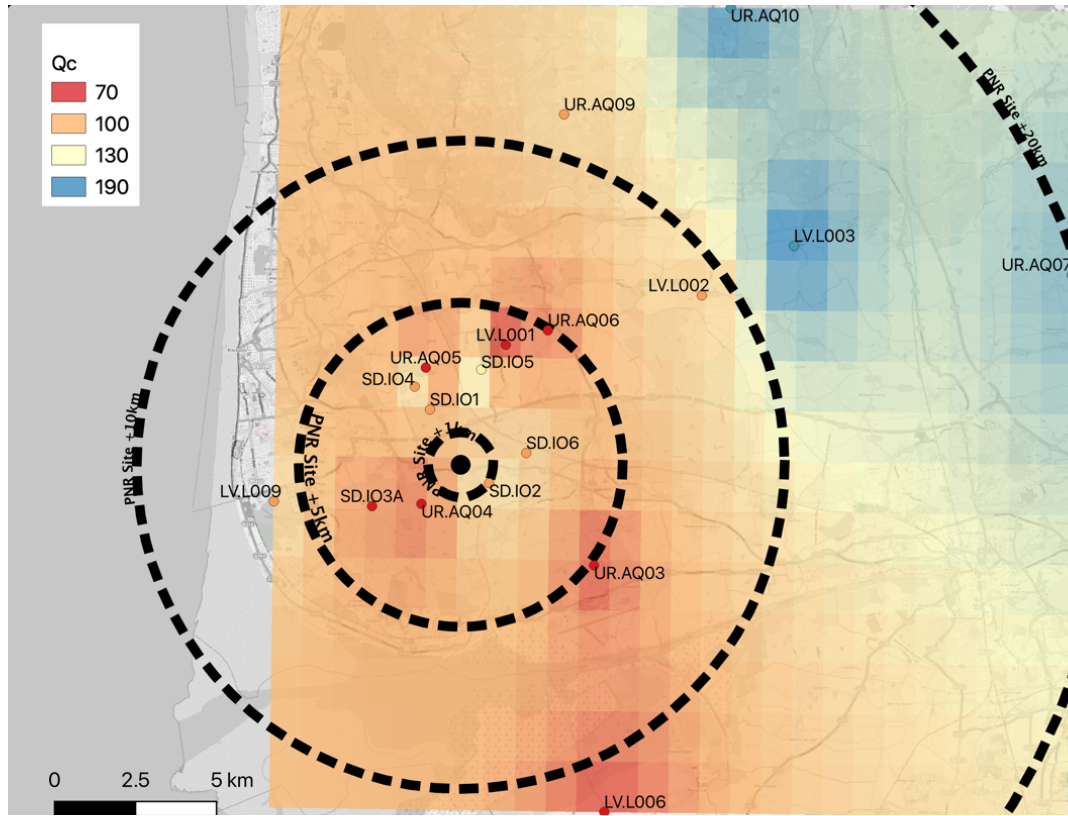


Figure 8 Map of $Q_{clt}(f=10)$ calculated using coda envelope decay method

Frequency-independent Q is model obtained from spectral fitting techniques using three different seismic phase windows: S-wave, coda wave, and S-coda wave windows. Three Q models from different seismic windows show similar results with $Q_s = 179.63$, $Q_c = 168.09$ and $Q_{sc} = 215.96$. Butcher et al. (2020) reported mean Q -value for the New Ollerton (UK) data (approx. 300 km from Preston New Road) is 116. This frequency-independent Q was calculated from 305 seismic events with magnitude ranging between $-0.7 < M_L < 2.1$ recorded from 7 broadband seismometers installed near New Ollerton, UK to investigate mining-induced seismicity. The amplitude spectrum of each individual events was generated and inverted for their best-fitting of Q by combining κ_0 with a Brune source model. Butcher at al. (2020) suggest that the spectral fitting method is more likely to give frequency-independent Q since intrinsic attenuation is dominant and the short signal windows do not really account for the influence of scattering, particularly at short distances.

d. Site Response

500 Site effects describe the local effect of uppermost layers of rocks and soil when the seismic
 501 waves propagating through them. The differences of the ground motion due to the Earth structure
 502 below the site can be related to different factors, in general, the main factor is the impedance
 503 contrast between the soil layers or soft sedimentary and the bedrock. The impedance contrast
 504 determines how strong the waves are at particular frequencies.

505 In general, there are two approaches to estimate the site effect using experimental methods:
 506 (1) reference site and (2) non-reference site techniques (Bard,1995). The reference site method
 507 (RSM) estimated by comparing records at the nearby sites, using one as the reference site. It is
 508 assumed that records from the reference site (in general a station installed on outcropping hard
 509 rock) contain the same source and propagation effects as records from the other sites. Therefore,
 510 differences observed between the sites are explained as being due to the local site effects. However,
 511 a major drawback of these methods is that a suitable reference site may not always be available.
 512 In order to overcome this disadvantage, non-reference site techniques such as the horizontal-to-
 513 vertical (H/V) spectral ratio method are widely used.

514 In this study, we examined H/V spectral ratios using ambient seismic noise (HVSR_N) also
 515 earthquake recordings (HVSR_E) at 17 sites (9 Liverpool sites, and 8 BGS sites). The result of
 516 HVSR_E calculated by taking the whole recordings for each event recorded from three different
 517 components. Apparently, the result giving a close value of resonance frequency (f_0) and peak
 518 amplification (A_0) obtained from HVSR_N. The resonance frequency as a result of HVSR method
 519 can be related to a simple model in terms of a layer over half-space that approximated as:

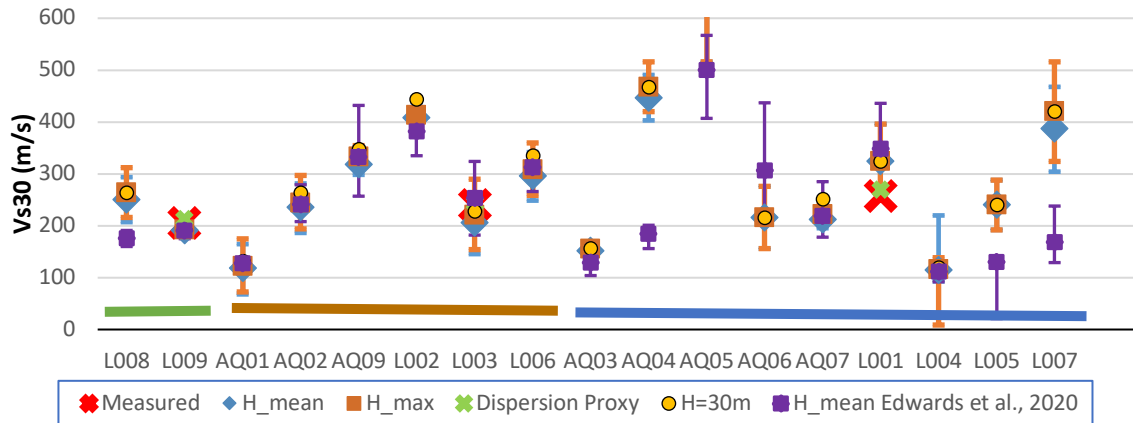
$$f_{r,0} = \frac{V_s}{4H_B} \quad (13)$$

520 where V_s is the shear waves velocity of the overlying layer and H_B is the depth of bedrock layer
 521 (Hassani and Atkinson, 2016). By taking the estimates of bedrock depth from BGS superficial
 522 deposits thickness model reported by Edwards et al. (2020), and assuming the bedrock shear-wave
 523 velocity (V_{sb}) as 1500 m/s, we can calculate V_{s30} as:

$$V_{s30} = \frac{30}{\frac{1}{4f_0} + \frac{\max(0, 30 - H_B)}{V_{sb}}} \quad (14)$$

524 A comparison of V_{s30} calculation from several observation that has been undertaken for PNR
 525 sites studies are presented in the Figure 9 and Table 2-3. Result from previous studies by Edwards
 526 et al.(2019, 2020) explained that site characterisation of PNR was performed using multi-channel
 527 analysis surface waves (MASW) at three different sites (L001, L003, and L009) and measurement
 528 of V_{s30} reported as 257 m/s for site L001, 240 m/s for site L003, and 205m/s for L009 (denote as
 529 red cross: measured V_{s30} in Figure 9). Besides measurement from shear-wave velocity profiles,
 530 estimation of V_{s30} also carried out using correlation of Rayleigh wave phase velocity dispersion of
 531 the 40-45 m wavelength signal (V_{s30} -dispersion proxy: shown as green cross in Figure 9).

532 The result of f_0 from our HVSRE observation then were used together with mean (H_{mean})
 533 (represented as blue diamond), and maximum bedrock depth (H_{max}) (denote as orange box) to
 534 calculate V_{s30} following Eq.14. Meanwhile, the f_0 from HVSRN represented by V_{s30} calculated
 535 by Edwards et al. (2020) using mean of bedrock depth (legend in Figure 9: “ H_{mean} Edwards et
 536 al.(2020)”). In addition, V_{s30} also estimated under the assumption of uniform bedrock at 30 m
 537 depth.



538
 539 *Figure 9 Estimated V_{s30} grouped by superficial geology (green: blown-sand, brown: peat, and*
 540 *blue: till). Error bars based on the lower and higher estimates of f_0 (very small error bar for*
 541 *“Measured V_{s30} ”, please see Table 3.)*

542 Wider variability of V_{s30} found in till ranging around 100- 600 m/s, while in peat ranging
 543 between 100-400 m/s and around 200-300 m/s for blown-sand. The estimated V_{s30} from f_0 are
 544 close to the measured values in three different sites (L001, L003, and L009). Compared to V_{s30}
 545 calculated using f_0 - HVSRN by Edwards et al. (2020), the calculated V_{s30} using f_0 from HVSRE
 546 is relatively larger at some sites (AQ04, AQ05, L005, and L007) which could be influenced by
 547 the quality of signal used in the HVSRE analysis.

Table 2 V_{s30} calculated using combinations of mean/ max bedrock depth and f_0 and its upper/ lower estimates.

		This study											
		f0 selected				Vs30 Estimates (m/s) - This Study							
Name	Net Geology	Bedrock Depth [Mean] (m)	Bedrock Depth [Max] (m)	f0 (Hz)	f0 low (Hz)	f0 high (Hz)	f0 Quality	Vs30 (H_mean, n, f0)	Δ Vs30 (H_mean, f0_low, f0_high)	Vs30 (H_max, f0)	Δ Vs30 (H_max, f0_low)	Δ Vs30 (H_max, f0_high)	
L008	UOL SAND	24	32	2.2	1.8	2.6	Clear	176	15	16	182	16	17
L009	UOL SAND	29	30	1.6	1.5	1.7	Clear	191	13	16	193	13	16
AQ01	BGS PEAT	4	11	1.1	0.6	1.6	Clear	127	11	11	131	11	11
AQ02	BGS PEAT	16	21	2.2	1.7	2.7	Clear	241	38	33	251	40	36
AQ09	BGS PEAT	22	26	2.9	2.7	3.1	Moderate	332	100	75	348	108	83
L002	UOL PEAT	24	25	3.7	3.4	4	Clear	382	64	47	387	66	48
L003	UOL PEAT	16	26	1.9	1.3	2.5	Moderate	254	70	72	278	82	88
L006	UOL PEAT	18	22	2.8	2.3	3.3	Clear	313	22	47	327	23	52
AQ03	BGS TILL	25	29	1.3	1.3	1.3	Clear	129	12	25	131	12	26
AQ04	BGS TILL	27	34	3.9	3.5	4.3	Broad	184	17	28	187	18	29
AQ05	BGS TILL	33	37	8.4	4.3	12.5	Broad	500	67	93	500	67	93
AQ06	BGS TILL	34	37	1.8	1.3	2.3	Broad	307	130	85	307	130	85
AQ07	BGS TILL	8	14	2.1	1.9	2.3	Moderate	219	66	41	229	71	45
L001	UOL TILL	31	36	2.7	2.1	3.3	Moderate	349	87	29	349	87	29
L004	UOL TILL	19	21	1.0	0.8	2.0	Broad	111	14	19	111	15	19
L005	UOL TILL	30	32	2	1.6	2.4	Broad	130	12	109	130	12	109
L007	UOL TILL	24	31	3.5	2.7	4.3	Broad	168	70	39	174	74	42

Table 3 V_{s30} calculated using combinations of mean/ max bedrock depth and f_0 and its upper/lower estimates, also V_{s30} measured, V_{s30} from dispersion, and V_{s30} ($H=30m$) from Edwards et al. (2020)

Edwards et al., (2020)																
Name	Net	Geology	Bedrock Depth [Mean] (m)	Bedrock Depth [Max] (m)	Measured V_{s30} (m/s) from $V_s(z)$ (mean)	Measured V_{s30} (m/s) from $V_s(z)$ (16th-percentile)	Measured V_{s30} (m/s) from $V_s(z)$ (84th-percentile)	V_{s30} (m/s) = VR 40-45 (Dispersion)	V_{s30} (m/s) uniform bedrock	f0 selected-HVSRN			Vs30 Estimates (m/s)			
										f0 (Hz)	f0 low (Hz)	f0 high (Hz)	f0 Quality	V_{s30} (H_{mean}, f_0)	ΔV_{s30} (H_{mean}, f_0_{low})	ΔV_{s30} (H_{mean}, f_0_{high})
L008	UOL	BLOWN SAND	24	32				264	1.52	1.39	1.66	Very Clear	176	15	16	
L009	UOL	BLOWN SAND	29	30	205.3	203.3	205.5	213	192	1.60	1.74	Very Clear	191	13	16	
AQ01	BGS	PEAT	4	11				132	1.19	1.08	1.30	Very Clear	127	11	11	
AQ02	BGS	PEAT	16	21				264	2.26	1.87	2.61	Clear	241	38	33	
AQ09	BGS	PEAT	22	26				348	3.04	2.06	3.81	Moderate	332	100	75	
L002	UOL	PEAT	24	25				444	3.45	2.83	3.91	Clear	382	64	47	
L003	UOL	PEAT	16	26	240	237.3	244.3	244	228	2.41	1.68	3.21	Moderate	254	70	72
L006	UOL	PEAT	18	22				336	2.99	2.75	3.51	Very Clear	313	22	47	
AQ03	BGS	TILL	25	29				156	1.10	1.00	1.32	Clear	129	12	25	
AQ04	BGS	TILL	27	34				468	1.56	1.41	1.80	Very Clear	184	17	28	
AQ05	BGS	TILL	33	37				1008	4.17	3.61	4.94	Clear	500	67	93	
AQ06	BGS	TILL	34	37				216	2.56	1.48	3.27	Broad	307	130	85	
AQ07	BGS	TILL	8	14				252	2.18	1.44	2.68	Moderate	219	66	41	
L001	UOL	TILL	31	36	257.1	251.7	264.4	269	324	2.91	2.18	3.15	Moderate	349	87	29
L004	UOL	TILL	19	21				120	0.96	0.83	1.13	Clear	111	14	19	
L005	UOL	TILL	30	32				240	1.08	0.98	1.99	Broad	130	12	109	
L007	UOL	TILL	24	31				420	1.45	0.84	1.80	Broad	168	70	39	

550 These V_{s30} result later on can be mapped and observed respected to the geology map to
 551 characterize the site effect of PNR area. Despite that there is no universal agreement that V_{s30} is a
 552 valid proxy to determine seismic amplification which appears to be too complex to be related to
 553 the V_s profile in the first 30 meters alone, certainly there is a correlation between V_{s30} and site
 554 amplification (Castellaro et al., 2008; Hartzell et al, 2001).

555 Aside from amplification effect of decreasing seismic velocity toward the surface, a
 556 counteractive effect of damping, $D(f)$, applies at high frequencies (Anderson and Hough, 1984).

$$D(f) = \exp(-\pi f \kappa_0) \quad (15)$$

557 which κ_0 is a site-specific damping term related to Q . This exponential decay in high-frequency
 558 energy is proposed to primarily reflect source-station attenuation and local site response (Anderson
 559 & Hough, 1984; Ktenidou et al., 2013; Neighbors et al., 2015; Parolai, 2015).

560 A variety of approaches are proposed for estimating κ_0 , the most commonly used is source
 561 spectra techniques (Ktenidou et al.,2014). Anderson and Hough, (1984) proposed an approach to
 562 estimate κ_0 using decay of the S-wave Fourier spectrum. In this study, κ_0 value obtained following
 563 spectral fitting method such that average path attenuation t^* can be given by $t^* = \kappa_0 +$
 564 $r_{hyp} / (Q\beta)$. The estimated κ_0 from this are study presented in Table 4.

565 *Table 4 Estimated κ_0 from spectral fitting method*

κ_0 (s) from this study			κ_0 (s) from Butcher et al. (2020)	
Signal window	Using all events	4 Biggest events only	Signal window	
S-wave window	0.01	0.013	Noise window	0.027
Coda window	0.017	0.01	Coda window	0.03
S-coda window	0.007	0.002	Direct wave window	0.025

566

567

568 V. Final Remarks and Future Studies

569 This study has important consequences for understanding ground motions from induced
 570 earthquake at Preston New Road, and for future studies of seismic hazard from these typical events.
 571 Ground motion prediction equations (GMPEs) can be used to characterize peak amplitudes and

572 response spectra as a function of magnitude, hypocentral distance, and other variables, for use in
573 seismic hazard analysis (e.g. Atkinson, 2015; Yenier et al., 2017; Atkinson and Assatourians,
574 2017). Such GMPEs are not yet well-developed for induced events. Several studies have been
575 carried out to develop GMPEs for induced seismicity (e.g. Atkinson, 2015), however, many of
576 them still use the assumption that induced events have source and attenuation parameters that are
577 broadly similar to those of natural earthquakes. Nevertheless, there is a significant systematic
578 discrepancy in source properties between natural and induced earthquake. Induced earthquakes
579 have systematically lower stress parameters than natural earthquakes (Hough, 2014). From the
580 scaling relationships of Boore (1983) and Hanks and Johnston (1992), we know that high
581 frequency ground motions depend strongly on the stress parameter and weakly on the moment
582 magnitude.

583 The Preston New Road (PNR) dataset with magnitudes < 3 at distances less than 30 km was
584 utilized in this study to better understand ground motion characteristics for induced seismicity and
585 to work toward the development of a GMPE specifically designed for the magnitude and distance
586 range of induced seismic events. Preliminary studies about GMPEs applied to data at PNR area
587 has been conducted by Edwards et al. (2019) by simply borrowing GMPEs from Atkinson (2015)
588 and Douglas et al. (2013) combined with magnitude model by Grünthal et al. (2009) and Edwards
589 et al. (2015). Those approach in fact s not the best solution and still has limitations such as the
590 unsatisfactory sigma. In order to avoid these limitations, we observe each different aspect (source,
591 path, and site term) as tools for developing new GMPEs.

592 The work presented in this deliverable provides key elements required for the development of
593 new GMPEs for PNR research area. Based on the study of attenuation parameters, in the case of
594 induced earthquakes which occur at shallow depth, the seismic energy decayed more rapidly. This
595 can be caused by physical properties that are quite different from the natural earthquakes. Hence,
596 the use of GMPEs that directly adopted from natural or moderate-to-larger earthquakes does not
597 represent the condition properly.

598 Further studies are underway to aid development of new GMPE specifically for induced
599 earthquakes. In particular: the analysis of source parameters (how the depth as well as earthquake
600 triggering mechanism could affect stress parameter); calibration and simulation based on physical
601 properties in accordance with the characteristic of induced seismicity; and analysing the potential
602 of regional differences in ground motion due to site effects by comparing with other regions.

603 Through a better understanding of ground motions and their controlling factors, future studies will
604 be able to draw robust conclusions on the behaviour of ground motions from these events and
605 reduce the uncertainty associated with GMPEs for induced events, such as those at Preston New
606 Road.

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